

Structure Point

CONCRETE SOFTWARE SOLUTIONS

sp slab

sp column

sp mats

sp beam

sp frame

sp wall

Work quickly.
Work simply.
Work accurately.

StructurePoint's Productivity Suite of powerful software tools for reinforced concrete analysis & design

sp wall

Finite element analysis & design of reinforced, precast ICF & tilt-up concrete walls

sp column

Design & investigation of rectangular, round & irregularly shaped concrete column sections

sp mats

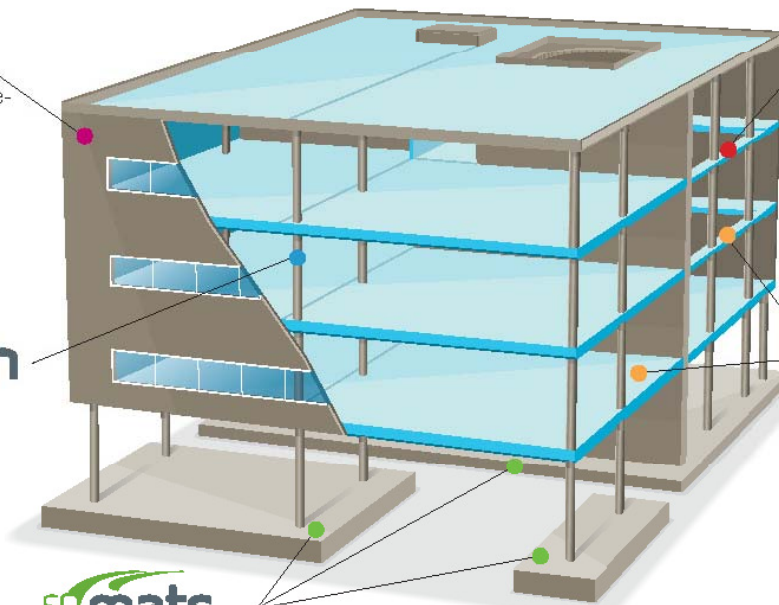
Finite element analysis & design of reinforced concrete foundations, combined footings or slabs on grade

sp beam

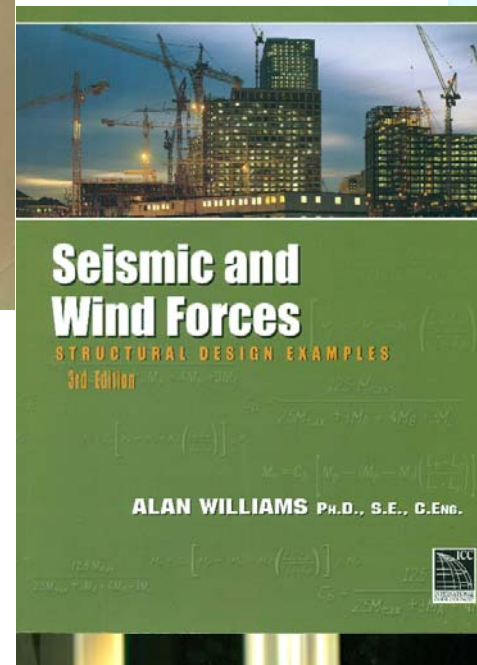
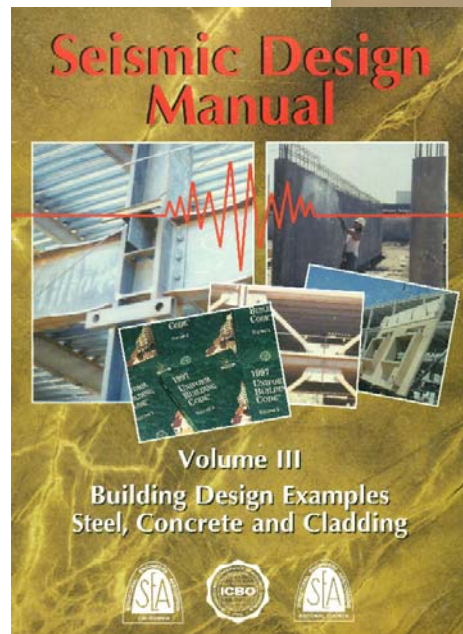
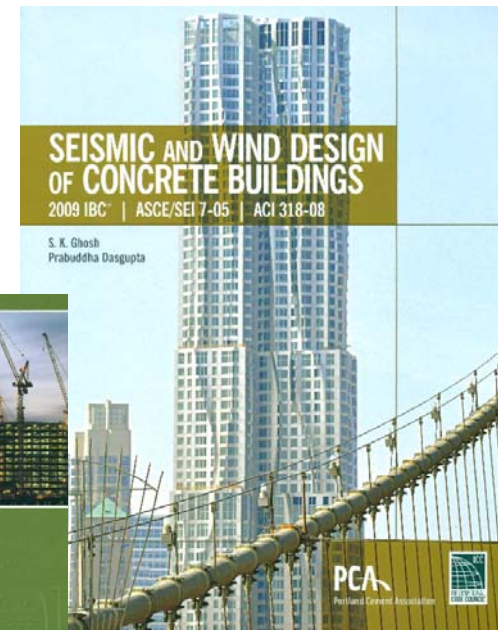
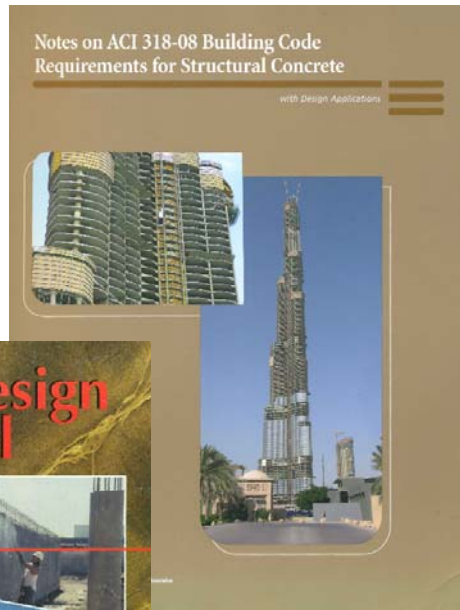
Analysis, design & investigation of reinforced concrete beams & one-way slab systems

sp slab

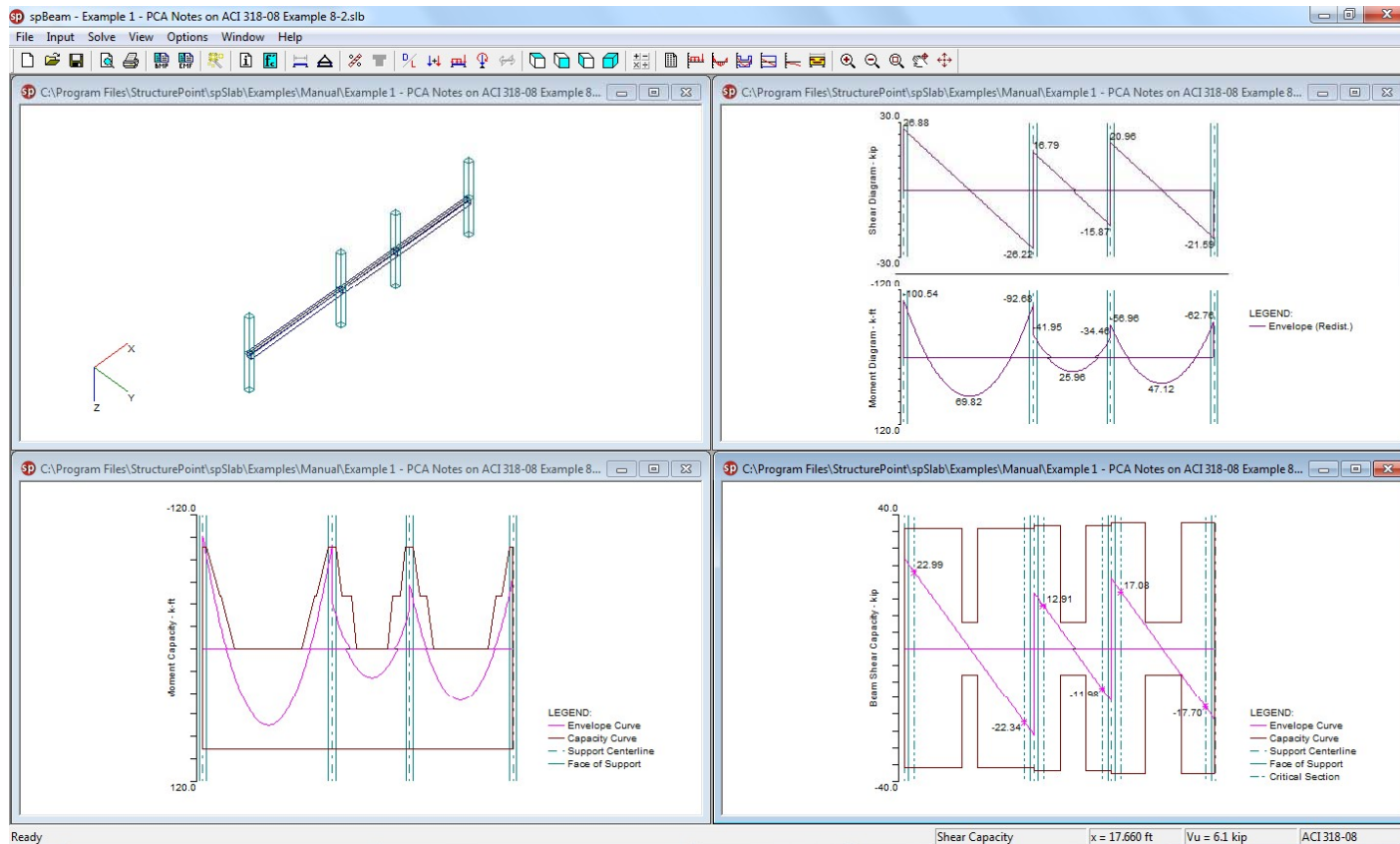
Analysis, design & investigation of reinforced concrete beams & slab systems



The Industry Standard



- Analysis, design, and investigation of R/C beams and one-way slab systems



Options



General Information

General Information | Span Control | Solve Options

Labels

Project: spSlab/spBeam Manual, Example 1

Frame: PCA Notes on ACI 318-08, Example 8-2

Engineer: StructurePoint

Options

Design code: ACI 318-08

Reinforcement: ASTM A615

Run mode

Design

Investigation

Frame

No. of Supports: 4

Left cantilever Right cantilever

Floor System

One-Way/Beam

Other

Distance location as ratio of span

OK Cancel Help

General Information

General Information | Span Control | Solve Options

Design Options

Live load pattern ratio: 100 %

Compression Reinforcement

Decremental Reinf. Design

Effective flange width

Rigid beam-column joint

Moment Redistribution

Torsion Analysis and Design

Torsion type

Equilibrium

Compatibility

Stirrups in flanges

No

Yes

Deflection calculation options

Sections to use in deflection calculations are

Gross (uncracked)

Effective (cracked)

In negative moment regions, to calculate I_g and M_{cr} use

Rectangular Section

T-Section

Calculate long-term deflections

Duration of load

60 months

Sustained part of live load

0 %

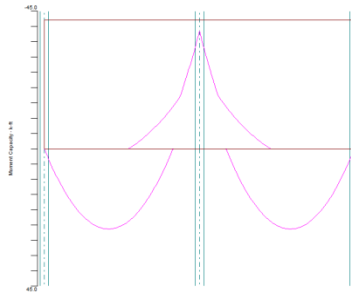
OK Cancel Help

Moment Redistribution



For ACI 318-08, 05, and 02

$$\delta = \begin{cases} 0, & \text{if } s_t < 0.0075 \\ 1000s_t, & \text{if } s_t \geq 0.0075 \end{cases} \text{ or}$$



Top Reinforcement

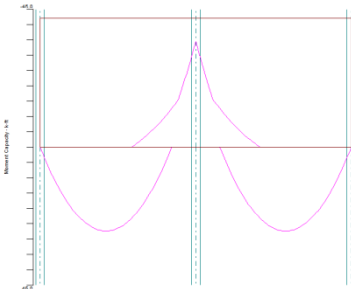
Units: Width (ft), Mmax (k-ft), Xmax (ft), As (in²), Sp (in)

Span Zone	Width	Mmax	Xmax	AsPrime	AsMin	AsMax	SpReq	AsReq	Bars
1 Left	2.08	0.00	0.667	0.000	0.000	1.056	9.792	0.000	3-#5 *5
Middle	2.08	3.70	15.400	0.000	0.094	1.056	9.792	0.071	3-#5 *3 *5
Right	2.08	33.18	23.333	0.000	0.195	1.056	9.792	0.706	3-#5
2 Left	2.08	33.18	0.667	0.000	0.195	1.056	9.792	0.706	3-#5
Middle	2.08	3.70	8.600	0.000	0.094	1.056	9.792	0.071	3-#5 *3 *5
Right	2.08	0.00	23.333	0.000	0.000	1.056	9.792	0.000	3-#5 *5

NOTES:
 *3 - Design governed by minimum reinforcement.
 *5 - Number of bars governed by maximum allowable spacing.

For ACI 318-99

$$\delta = \begin{cases} 0, & \text{if } (\rho - \rho') > 0.5\rho_b \\ 20\left(\frac{\rho - \rho'}{\rho_b}\right), & \text{if } (\rho - \rho') \leq 0.5\rho_b \end{cases} \text{ or}$$



Moment Redistribution Factors

Units: Org.Mu (k-ft)

Supp	Side	Calculated				User Limit[%]	Applied Factor[%]
		Org.Mu	Iter.#	EpsilonT	Factor[%]		
1	Right	0.00	0	0.00000	0.00	0.00	0.00
2	Left	33.18	5	0.00998	9.98	10.00	9.98
2	Right	33.18	5	0.00998	9.98	10.00	9.98
3	Left	0.00	0	0.00000	0.00	0.00	0.00

Top Reinforcement

Units: Width (ft), Mmax (k-ft), Xmax (ft), As (in²), Sp (in)

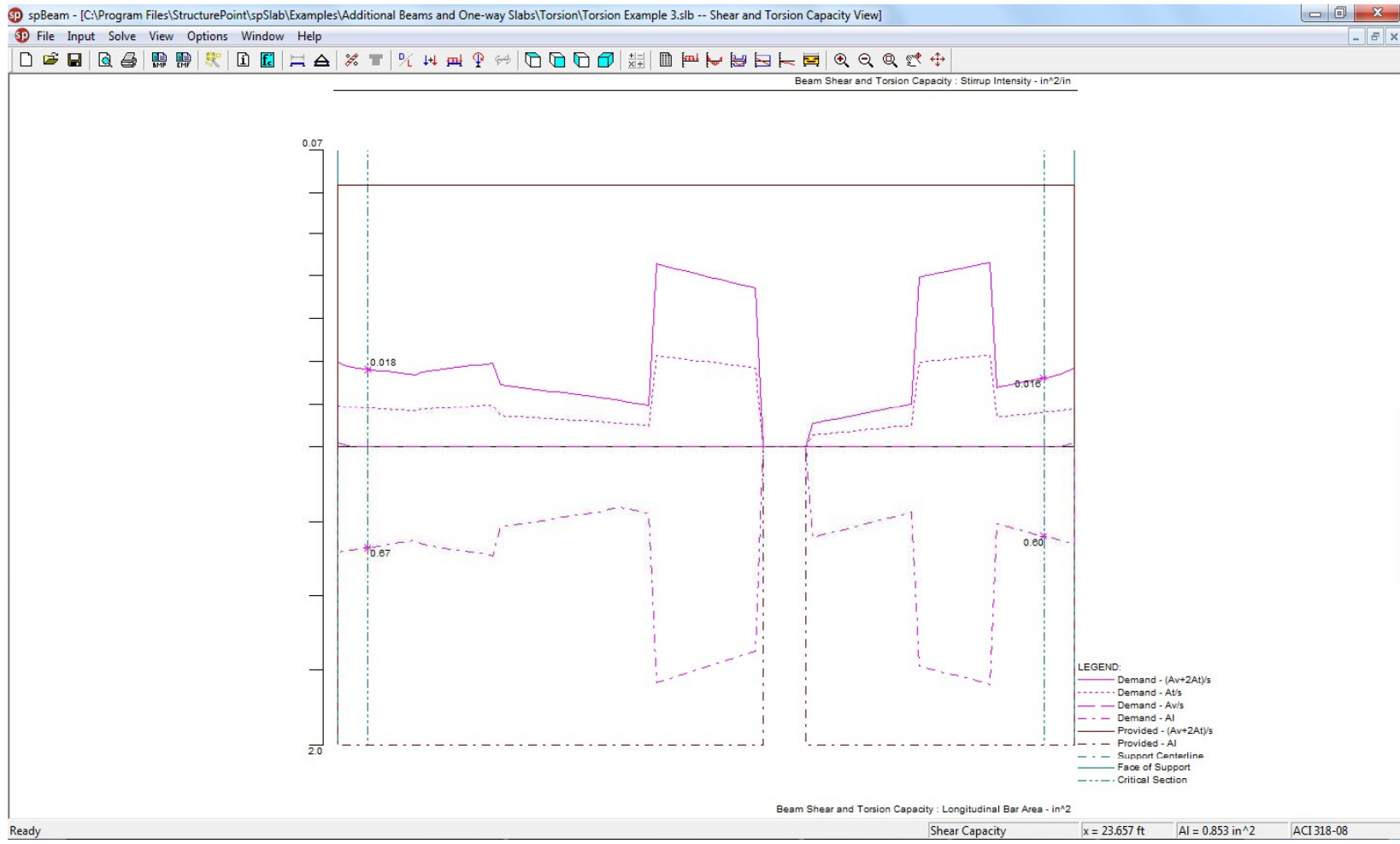
Span Zone	Width	Mmax	Xmax	AsPrime	AsMin	AsMax	SpReq	AsReq	Bars
1 Left	2.08	0.00	0.667	0.000	0.000	1.056	9.792	0.000	3-#5 *5
Middle	2.08	2.01	15.400	0.000	0.082	1.056	9.792	0.038	3-#5 *3 *5
Right	2.08	29.46	23.333	0.000	0.195	1.056	9.792	0.618	3-#5 *5
2 Left	2.08	29.46	0.667	0.000	0.195	1.056	9.792	0.618	3-#5 *5
Middle	2.08	2.01	8.600	0.000	0.082	1.056	9.792	0.038	3-#5 *3 *5
Right	2.08	0.00	23.333	0.000	0.000	1.056	9.792	0.000	3-#5 *5

NOTES:
 *3 - Design governed by minimum reinforcement.
 *5 - Number of bars governed by maximum allowable spacing.

For CSA A23.3

$$\delta = 30 - 50\frac{c}{d}$$

Shear and Torsion





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Call: +1-847-966-4357

Email: info@StructurePoint.org