

# StructurePoint

CONCRETE SOFTWARE SOLUTIONS

sp **slab**

sp **beam**

sp **column**

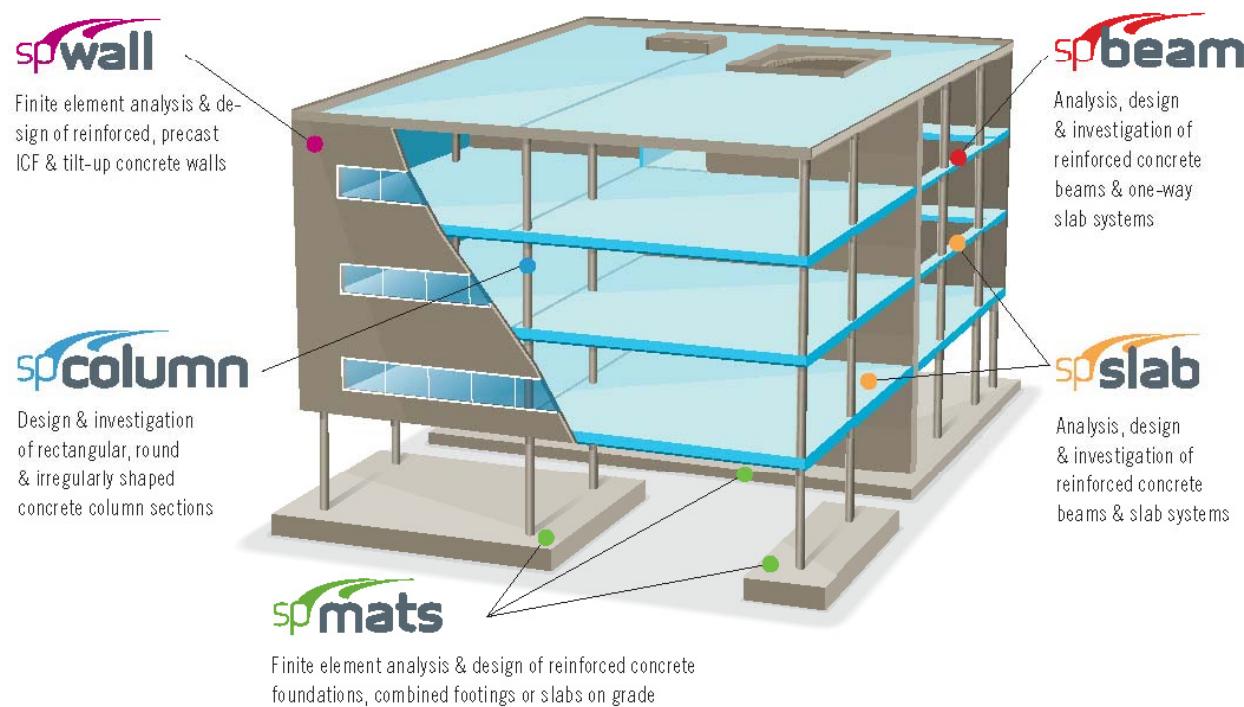
sp **frame**

sp **mats**

sp **wall**

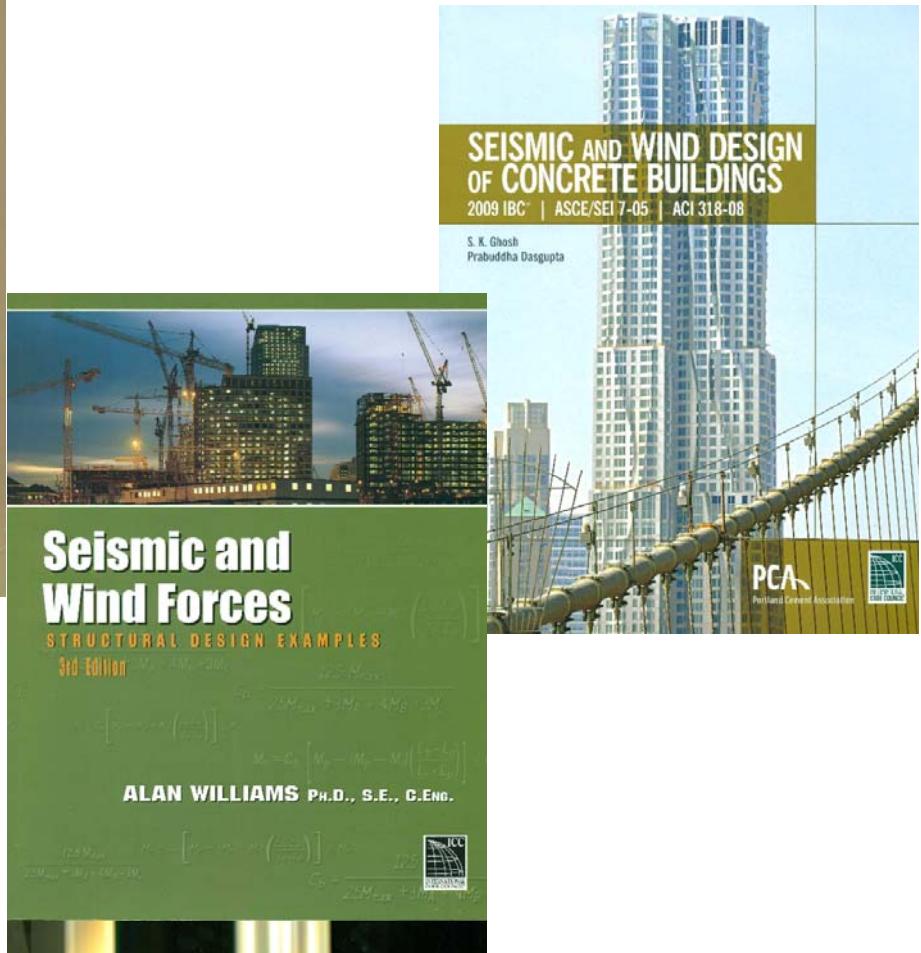
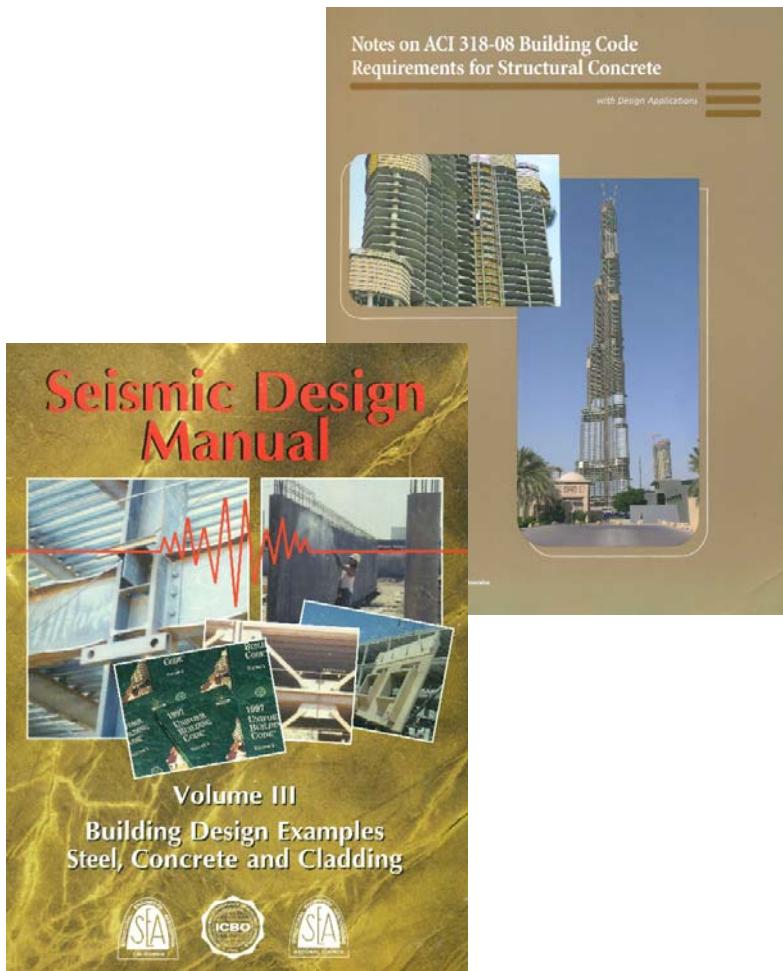


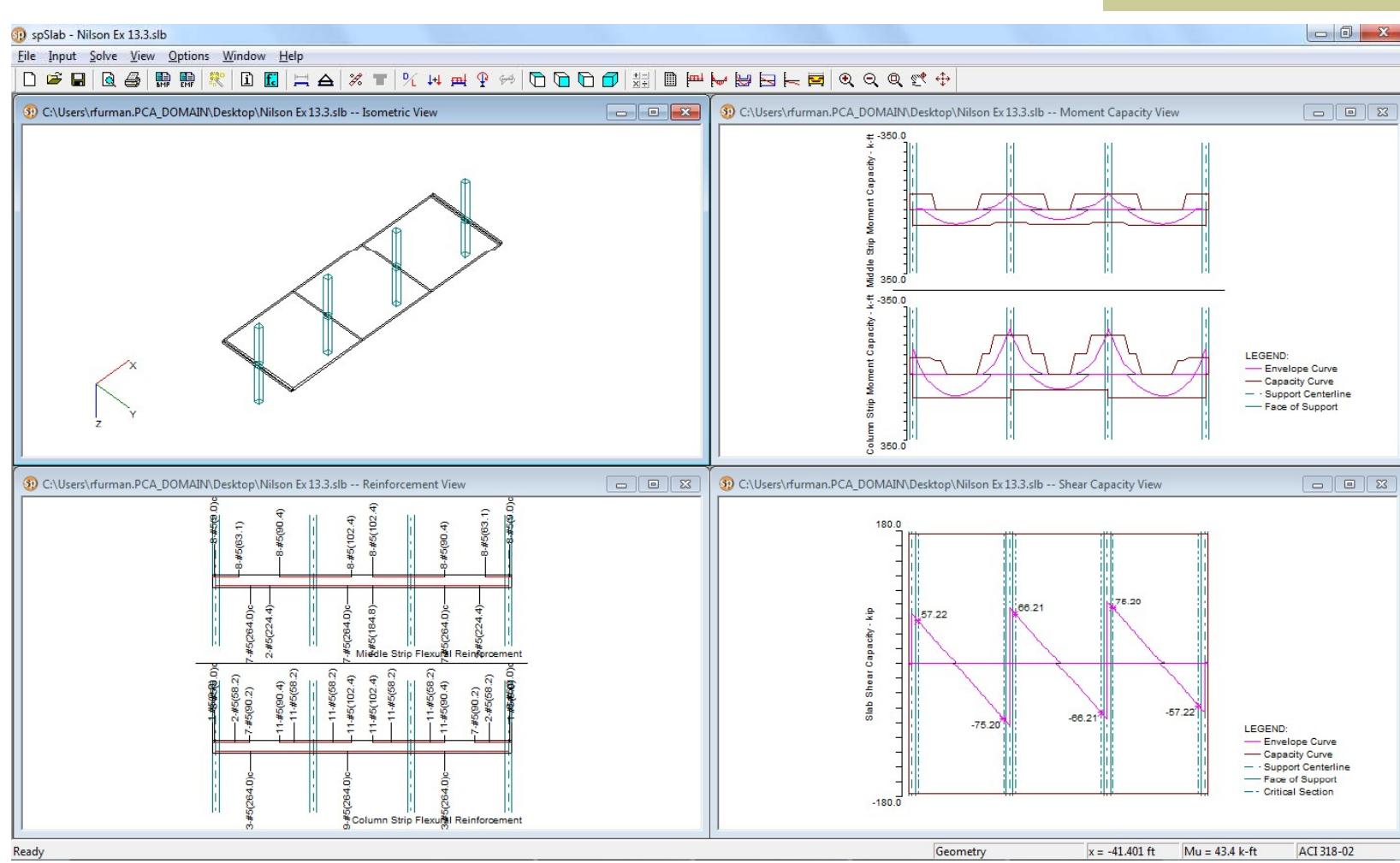
## StructurePoint's Productivity Suite of powerful software tools for reinforced concrete analysis & design



# The Industry Standard

spcolumn





# Options



**General Information**

General Information | Span Control | Solve Options

Labels

Project: Demonstration Design

Frame:

Engineer:

Options

Design code: ACI 318M-08

Reinforcement: ASTM A615M

Run mode

Design

Investigation

Frame

No. of Supports: 2

Left cantilever  Right cantilever

Floor System

Two-Way

One-Way/Beam

Other

Distance location as ratio of span

OK Cancel Help

Two-way systems

**General Information**

General Information | Span Control | Solve Options

Design Options

Live load pattern ratio: 100 %

Compression Reinforcement  User Slab Strip Widths

Decremental Reinf. Design  User Distribution Factors

Beam T-Section Design

One-way Shear In Drop Panels  Long. Bm. Supt. Design

Distribute Shear to Slab Strips  Trans. Bm. Supt. Design

Critical section for punching shear

Ignore side on a free edge if within 4 times the slab thickness from the face of the support.

Deflection calculation options

Sections to use in deflection calculations are

Gross (uncracked)  Effective (cracked)

In negative moment regions, to calculate Ig and Mcr use

Rectangular Section  T-Section

Calculate long-term deflections

Duration of load 60 months Sustained part of live load 0 %

OK Cancel Help

One-way systems

**General Information**

General Information | Span Control | Solve Options

Design Options

Live load pattern ratio: 100 %

Compression Reinforcement  Effective flange width

Decremental Reinf. Design  Rigid beam-column joint

Beam T-Section Design  Moment Redistribution

Torsion Analysis and Design

Torsion type

Equilibrium  Stirrups in flanges

Compatibility  No

Compatibility  Yes

Deflection calculation options

Sections to use in deflection calculations are

Gross (uncracked)  Effective (cracked)

In negative moment regions, to calculate Ig and Mcr use

Rectangular Section  T-Section

Calculate long-term deflections

Duration of load 60 months Sustained part of live load 0 %

OK Cancel Help

# Span Data



## Defining Spans

Span Data

Slabs/Flanges | Longitudinal Beams | Ribs |

Span:  Length:  ft Width Left:  ft  
Location:  Thickness:  in Width Right:  ft

Modify Copy...

Span No.	Location	Length	Thickness	Width-L	Width-R
1	Interior	0.7	7	7	7
2	Interior	18	7	7	7
3	Interior	18	7	7	7
4	Interior	18	7	7	7
5	Interior	0.7	7	7	7

OK Cancel Help

## Span Manipulation

General Information

General Information | Span Control | Solve Options |

Support Selection  Left Support  Right Support

State

Reset All  
Restore  
Delete

Span Control List

New#	Old#	Sup L/R	Copy
1-CL	1-CL	- / 1	
2	2	1 / 2	
3	3	2 / 3	
4	4	3 / 4	
5-CR	5-CR	4 / -	

Insert

Before <- After ->

Copy

Before <- After ->

Move

Before <- After ->

OK Cancel Help

# Support Data



## Defining Supports

Support Data

Columns | Drop Panels | Column Capitals | Transverse Beams | Boundary Conditions |

Support:	1	Height (ft)	c1 (in)	c2 (in)					
Above:	9	16	16						
Below:	9	16	16						
Stiffness share %:	100	<input checked="" type="checkbox"/> Check punching shear around column <input type="checkbox"/> Increase GammaF							
<b>Modify</b> <b>Copy...</b>									
Sup. ...	Stiff %	HtA	c1A	c2A	HtB	c1B	c2B	Shear	Gamma
1	100	9	16	16	9	16	16	Yes	No
2	100	9	16	16	9	16	16	Yes	No
3	100	9	16	16	9	16	16	Yes	No
4	100	9	16	16	9	16	16	Yes	No

## Boundary Conditions

Support Data

Columns | Drop Panels | Column Capitals | Transverse Beams | Boundary Conditions |

Support:	1	Support Springs	Far End	
Above:	9	Vertical Kz: 0 kip/in	Column Above: Fixed	
Below:	9	Rotation Kry: 0 kip-in/rad	Column Below: Fixed	
<b>Modify</b> <b>Copy...</b>				
Sup. No	Kz	Kry	Far End - Above	Far End - Below
1	0	0	Fixed	Fixed
2	0	0	Fixed	Fixed
3	0	0	Fixed	Fixed
4	0	0	Fixed	Fixed

# Reinforcement



## ■ Design

Reinforcement Criteria

Slabs and Ribs | Beams

Cover (in)

Top bars	Bottom bars
Clear: 1.5	1.5

Bar size

Min: #4	#4
Max: #4	#4

Spacing (in)

Min: 1	1
Max: 10	10

Reinf. ratio (%)

Min: 0.18	0.18
Max: 2	2

There is more than 12 in of concrete below top bars.

OK Cancel Help

## ■ Investigation

Reinforcing Bars

Column Strip Bars | Middle Strip Bars | Beam Bars | Beam Stirrups

Span 1  
Span 2  
Span 3  
**Span 4**  
Span 5

Bar size: #5 No. of bars: 11 Length (ft): 7.5311

Top left Cover (in): 1.125

Span = 22 ft

Span Copy... Add Modify Delete

Size	Type	Count	Cover	Length	Start
#5	TopL	11	1.125	7.53112	--
#5	TopL	11	1.125	4.8506	--
#5	TopR	7	1.125	7.51534	--
#5	TopR	2	1.125	4.8506	--
#5	BotC	13	1.125	--	--

OK Cancel Help

# Loads



**Load Combinations**

	SELF	Dead	Live	Snow	Wind	EQ
U1	1.4	1.4	0	0	0	0
U2	1.2	1.2	1.6	0.5	0	0
U3	1.2	1.2	1	1.6	0	0
U4	1.2	1.2	0	1.6	0.8	0
U5	1.2	1.2	0	1.6	-0.8	0
U6	1.2	1.2	1	0.5	1.6	0
U7	1.2	1.2	1	0.5	-1.6	0
U8	0.9	0.9	0	0	1.6	0
U9	0.9	0.9	0	0	-1.6	0
U10	1.2	1.2	1	0.2	0	1
U11	1.2	1.2	1	0.2	0	-1
U12	0.9	0.9	0	0	0	1
U13	0.9	0.9	0	0	0	-1

Add    Modify    Delete

**Support Loads and Displacements**

Current Case:	Support:	Displacement/Rotation	Force/Moment:
Dead Live	1	Dz: 1.5 in    Ry: 0 rad	Fz: 0 kip    My: 0 k-ft

Modify    Copy...

**Span Loads**

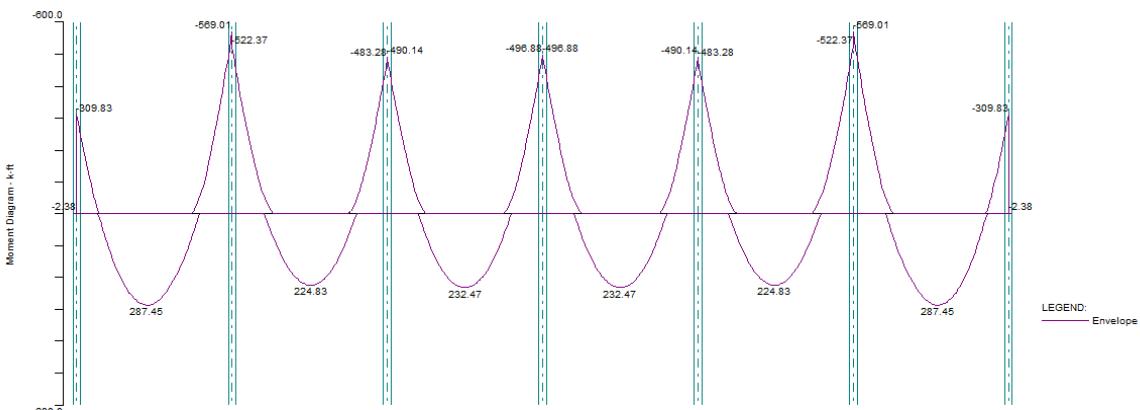
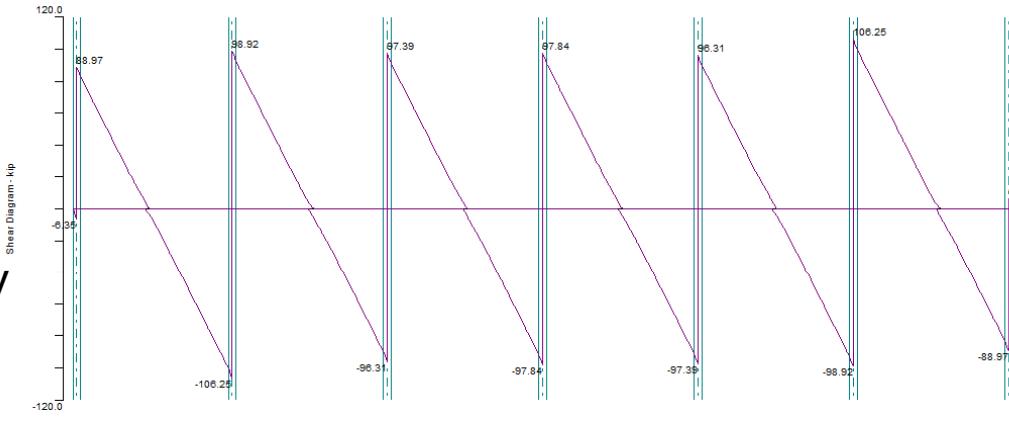
Current Case:	Span:	Magnitude:	Type:		
Dead Live	1	20	Area Load	lb/ft <sup>2</sup>	
Span = 0.7 ft					
Case Copy...		Add	Modify	Delete	

Span No.	Type	Wa	La	Wb	Lb
1	Area Load	20	-	-	-
2	Area Load	20	-	-	-
3	Area Load	20	-	-	-
4	Area Load	20	-	-	-
5	Area Load	20	-	-	-

Cancel    Help

# Graphical Output

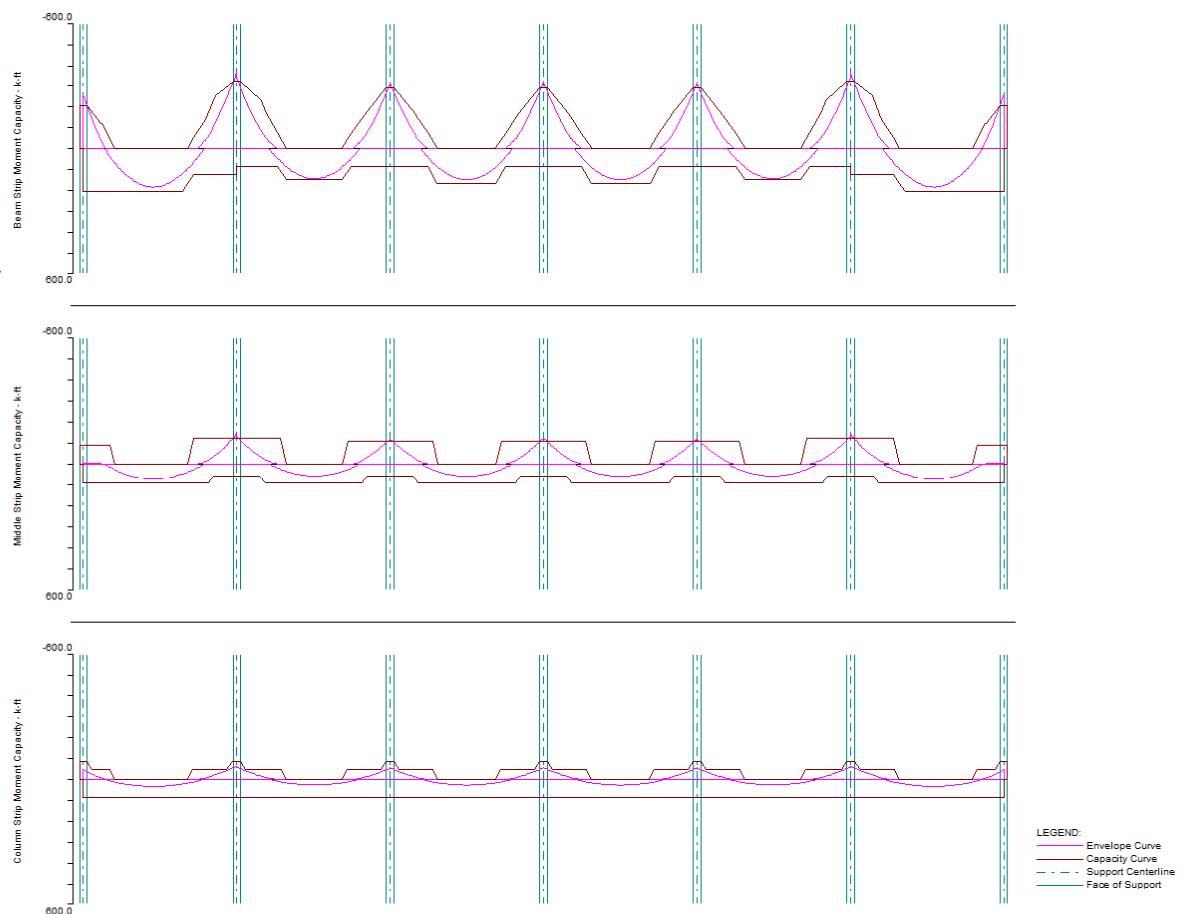
- Loads
- Internal Forces
- Moment Capacity
- Shear Capacity
- Deflections
- Reinforcement



## Internal Force Diagrams

# Graphical Output

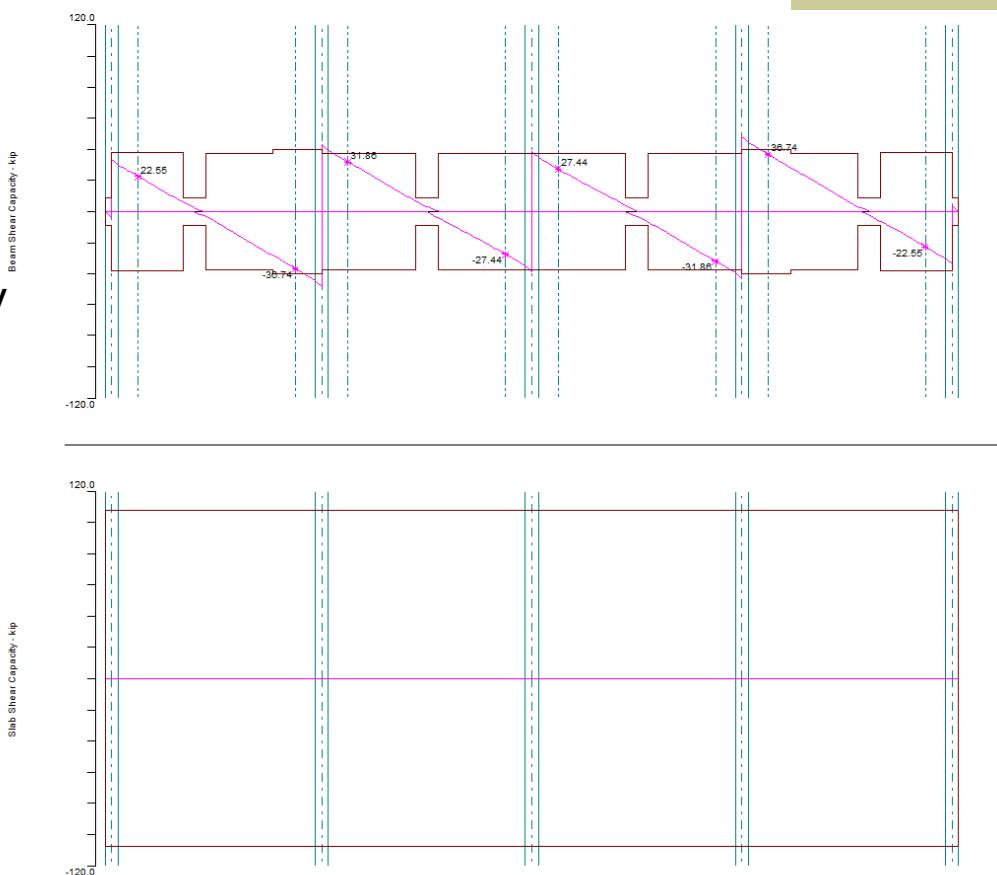
- Loads
- Internal Forces
- Moment Capacity
- Shear Capacity
- Deflections
- Reinforcement



Moment Capacity Diagrams

# Graphical Output

- Loads
- Internal Forces
- Moment Capacity
- Shear Capacity
- Deflections
- Reinforcement

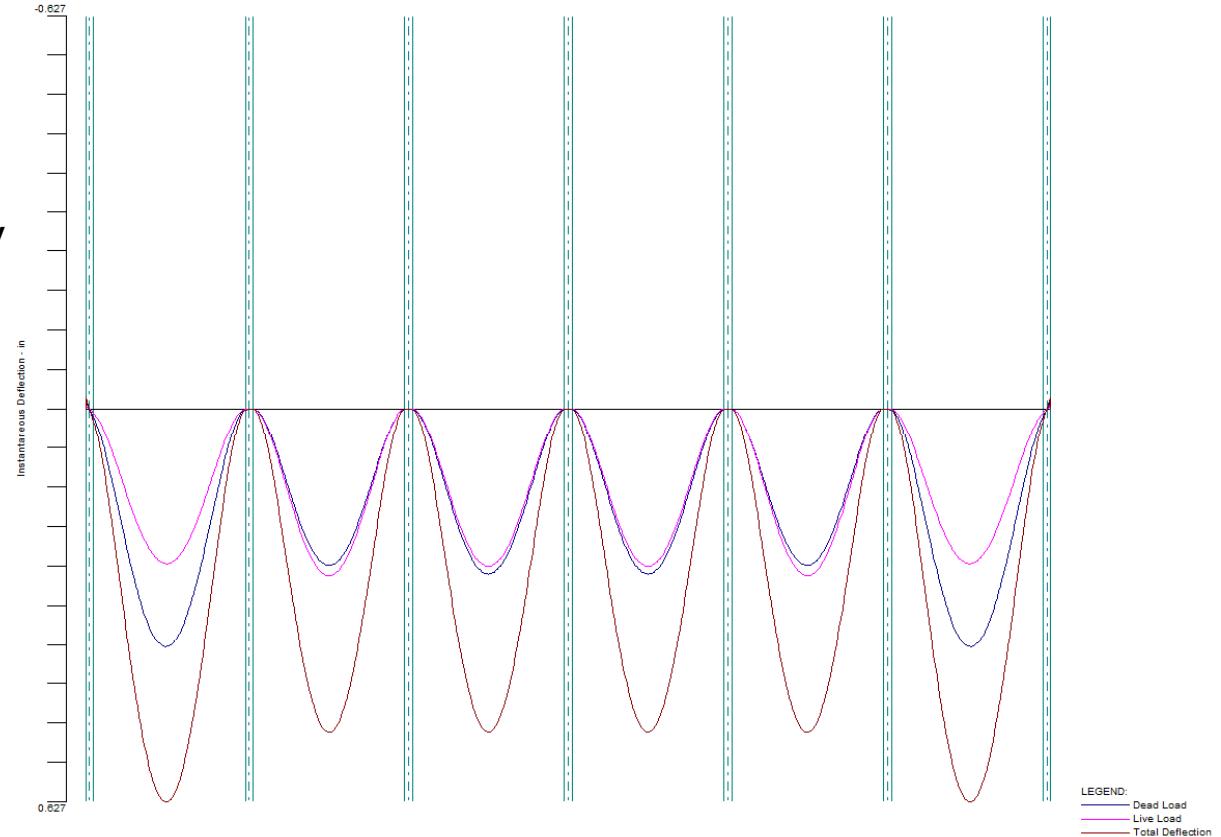


Shear Capacity Diagrams

# Graphical Output



- Loads
- Internal Forces
- Moment Capacity
- Shear Capacity
- Deflections
- Reinforcement



Deflection Diagram

# Text Output

- Input Echo
- Design Results
- Column Forces
- Internal Forces
- Deflections

**Results Report**

Close Select All Copy Print... [2] Design Results 551 lines

[1] Input Echo  
[2] Design Results  
[3] Column Axial Forces and Moments  
[4] Internal Forces - Load Cases  
[5] Internal Forces - Load Combinations  
[6] Internal Forces - Envelopes  
[7] Deflections  
[8] Required Reinforcement

**Deflections**

Section properties

Units: Ig, Icr, Ie (in^4), Mcr, Mmax (kip-in)

Span	Load Level		
	Dead	Ie, avg	Dead+Live
1	9333	9333 Right	9333
2	20489	20338 Left	8345
		Middle	36.89
		Right	-0.52
3	20421	20348 Left	2625
		9333	63.14
		Right	27.19
4	20489	20338 Left	8547
		9333	36.89
		Right	-58.35
5	9333	9333 Left	8547
		9333	36.89
		Right	-30.61
		9333	-0.52

Maximum Instantaneous Deflections - Direction of Analysis

Units: D (in), Ig (in^4)

Span	Frame			Strips		
	Ddead	Dlive	Dtotal	Strip	Ig	LDF
1	-0.001	-0.001	-0.003	Column	20040.5	0.781
			Middle	2862	0.219	
2	0.012	0.014	0.026	Column	20040.5	0.693
			Middle	2862	0.307	
3	0.006	0.007	0.013	Column	20040.5	0.673
			Middle	2862	0.327	
4	0.012	0.014	0.026	Column	20040.5	0.693
			Middle	2862	0.307	
5	-0.001	-0.001	-0.003	Column	20040.5	0.781
			Middle	2862	0.219	

Maximum Long-term Deflections - Direction of Analysis

Time dependant factor for sustained loads = 2.000

Units: D (in)

Span	Column Strip					Middle Strip						
	Dsust	Lambda	Dcs	Dcs+lu	Dcs+l	Dtotal	Dsust	Lambda	Dcs	Dcs+lu	Dcs+l	Dtotal
1	-0.001	2.000	-0.002	-0.004	-0.004	-0.005	-0.002	2.000	-0.005	-0.007	-0.007	-0.010
2	0.010	2.000	0.021	0.033	0.033	0.044	0.032	2.000	0.065	0.103	0.103	0.136
3	0.005	2.000	0.010	0.016	0.016	0.021	0.017	2.000	0.034	0.054	0.054	0.071
4	0.010	2.000	0.021	0.033	0.033	0.044	0.032	2.000	0.065	0.103	0.103	0.136
5	-0.001	2.000	-0.002	-0.004	-0.004	-0.005	-0.002	2.000	-0.005	-0.007	-0.007	-0.010

Material Takeoff

Reinforcement in the Direction of Analysis

Total Weight: 0.000 4.16 1.21 1.18 0.00 1.16 / 44.62



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